

Globe Engineering, Inc.

CIVIL ENGINEERING

LAND PLANNING

CONSTRUCTION MANAGEMENT

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FBPE registry # 8604

STORM DRAINAGE CALCULATIONS FOR Belmont Park Estate

NE corner of Pompano Park Place &
SW 4th Avenue

Pompano Beach, FL

Revised 05/01/2023

Digitally signed
by Martin

Pilote

Date:

2023.05.04

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Martin Pilote, State of Florida, Professional Engineer, License No. 55992

This item has been digitally signed and sealed by Martin Pilote, PE on the date indicated here.

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Martin Pilote
FLA. PE # 55992

DRC

PZ22-12000014

06/21/2023

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Revised July 22, 2022

**Storm Drainage Summary for
Belmont Park Estates
NE corner of Pompano Park Place & SW 4th Avenue
Pompano Beach, FL**

INTRODUCTION

We are proposing a new multi-family project on previously developed property located at the NE corner of Pompano Park Place & SW 4th Avenue, Pompano Beach, Florida.
The current property consists of vacant land. The total project area is 0.936 acre.

EXISTING SURFACE WATER MANAGEMENT SYSTEM

The existing site is vacant land. The existing site grades are shown on the survey.
The current groundwater elevation from Broward County is 2.50 ft NAVD.
The future groundwater elevation from Broward County is 3.00 ft NAVD.
The future groundwater elevation is used in the drainage calculations.

One percolation test was performed in accordance with SFWMD procedures, which provide an average hydraulic conductivity as shown in the attached Exfiltration Trench calculations. Please refer to the attached Geotechnical Report (perc tests and soil borings).

PROPOSED SURFACE WATER MANAGEMENT SYSTEM

The proposed drainage improvements for the site consist of a series of inlets with exfiltration trench that connect to dry retention areas, swales, and R-Tanks under the parking lot.

Water quality treatment is provided within the exfiltration trench. Please refer to the attached Exfiltration Trench calculations.

Since the post-development stages are higher than the pre-development stages for the 100-year/72-hour and 25-year/72 hours storm events, the project is required to retain the post-development runoff on-site for the 25-year/72 hours storm event.

LAND USE AND SOIL STORAGE

Refer to the attached drainage calculations for the existing and proposed Land Use and soil storage for the entire site.

FLOOD PLAIN

The 2014 FEMA flood zone is X for the most of the parcel, except for a small portion on the west side that is within FEMA flood zone AH7

For this site, the Broward County 100 year – 3 day zero discharge map is 8.50 ft

FINISHED FLOOR ELEVATION

The Finished Floor Elevation (FFE) will be 8.50 ft per the County's 100 year – 3 day zero discharge map.

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11.5

8.5

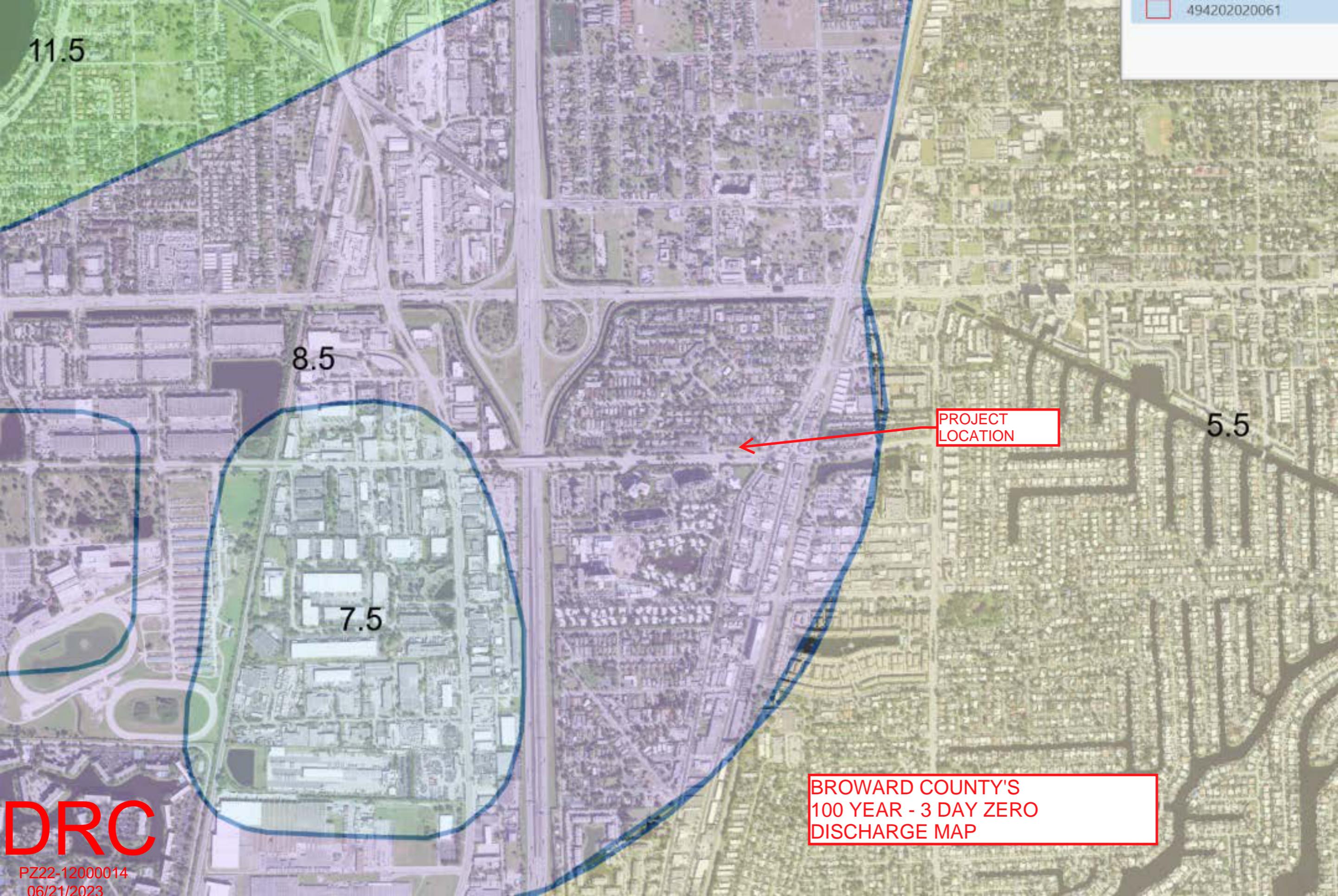
7.5

5.5

PROJECT
LOCATION

BROWARD COUNTY'S
100 YEAR - 3 DAY ZERO
DISCHARGE MAP

DRC



DATE: 4/16/2022

REV: 5/1/2023

BY: DJC

Belmont

POST DEVELOPMENT STORMWATER MANAGEMENT CALCULATIONS

LAND USE BREAKDOWN			GRADING PARAMETERS			
LAND USE BREAKDOWN	ACRES	PERCENT %	V/L	START ELEV	END ELEV	CHANGE
Building	0.130	13.9%	V	8.50	8.50	0.00
	0.000	0.0%	V	0.00	0.00	0.00
Pavement	0.326	34.8%	L	6.50	7.10	0.60
	0.040	4.3%	L	6.50	7.52	1.02
Sidewalk	0.063	6.8%	L	6.40	8.42	2.02
Green	0.265	28.3%	L	7.00	8.40	1.40
DRA Side	0.085	9.1%	L	4.00	7.00	3.00
DRA Bottom	0.0267	2.9%	V	4.00	4.00	0.00
	0.000	0.0%	V	0.00	0.00	0.00
TOTAL	0.936	100.0%				

Exfiltration Trench

3.28" x site area allowed in trench (Vtotal): 3.07 ac-in.
 Vtotal = Vwq + Vadd
 Vwq (see calcs on following pages): 1.25 ac-in
 %WQ: 0.5
 %WQ x Vwq: 0.623 ac-in
 Vadd max: 1.824 ac-in.
 FS: 2

Exfiltration Trench

$$L_{max} = \frac{FS(\%WQ)(Vwq) + Vadd}{[K \times (H_2W + 2H_2D_u - D_u^2 + 2H_2D_s) + (1.39 \times 10^{-4})WD_u]}$$

= 293.50 feet
 Lactual = 400.00

Vwq = 1.25 ac-in. (from above)
 $V_{addactual} = [L \times (K \times (H_2W + 2H_2D_u - D_u^2 + 2H_2D_s) + (1.39 \times 10^{-4})WD_u)] / 2 - FS(\%WQ \times Vwq)]$
 = 2.7 ac-in = **0.226 ac-ft**
 Vtotal actual 3.3 ac-in. **0.278 ac-ft**

Vtotal in stage storage computation (ac-in) = 3.334 ac-in
 = 0.278 ac-ft

$$V(ac-in) = L \times [K \times (H_2W + 2H_2D_u - D_u^2 + 2H_2D_s) + (1.39 \times 10^{-4})WD_u]$$

= 3.3 = 0.28 ac-ft **5 yr-1 hr**

Exfiltration Trench Stage-Storage Calcs. (Assuming Linear Progression)	
Stage (ft)	Storage (ac-ft)
3	0.00
3.5	0.05
4	0.10
4.5	0.15
5	0.20
5.5	0.25

Max. Storage Credit: **0.25 ac-ft**

Trench Characteristics
 L= 400 ft
 W= 10 ft
 K (CFS/ft²)= 3.02E-04
 H₂= 2.50 ft
 D_u= 2.50 ft
 D_s= 2.50 ft
 Top of Trench Elev.= 5.50 ft-NAVD
 Bottom of Trench Elev.= 0.50 ft-NAVD
 Dry Retention Rim Elev.= 5.50 ft-NAVD



STAGE-STORAGE CALCULATIONS											
Assume Linear Progression for all Areas											
STAGE (ft-NAVD)	Volume of Storage (ac-ft)										TOTAL
	Building	0	Pavement	Sidewalk	High Pavement	Green	DRA Side	DRA Bottom	R-tank	Exfil. Trench	
3.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000
3.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.04	0.05	0.086
4.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.10	0.171
4.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.11	0.15	0.273
5.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.03	0.14	0.20	0.383
5.50	0.00	0.00	0.00	0.00	0.00	0.00	0.03	0.04	0.18	0.25	0.500
6.00	0.00	0.00	0.00	0.00	0.00	0.00	0.06	0.05	0.21	0.25	0.573
6.50	0.00	0.00	0.00	0.00	0.00	0.00	0.09	0.07	0.21	0.25	0.619
7.00	0.00	0.00	0.07	0.01	0.00	0.00	0.13	0.08	0.21	0.25	0.750
7.50	0.00	0.00	0.23	0.02	0.02	0.02	0.17	0.09	0.21	0.25	1.017
8.00	0.00	0.00	0.39	0.04	0.04	0.09	0.21	0.11	0.21	0.25	1.348
8.50	0.00	0.00	0.55	0.07	0.06	0.21	0.26	0.12	0.21	0.25	1.733

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POST DEVELOPMENT STORMWATER MANAGEMENT CALCULATIONS

Project Location Hydraulic Details

Design Water Level= 3.00
 Allowable Discharge* = N/A csm

Design Storm Rainfall Amounts

Storm Frequency	Rainfall (in.)		
	1 hr	24 hr	72 hr
3 year	x	x	
5 year	3.2	x	
10 year		9.00	13.00
25 year			16.00
100 year			20.00

Soil Storage Calculation

- A. Total Pervious Area = 0.38 acres = 40.3%
- B. Depth to Water Table = 3.9 feet
- From SFWMD Permit Information Manual, Vol. IV, Figure E-1, For 'flatwoods', the Cumulative
- C. Available Soil Storage is:
 - Sp = 6.59 inches
- D. Site Soil Storage (SSS) = Sp x (Pervious Area/Total Area) = 2.65 inches
- E. Curve No. (CN) = $1000 / (SSS + 10) = 79$

Water Quantity Calculations

- A. Calculate the Runoff in Inches.
 - $Q = \frac{(\text{Rainfall} - 0.2 \times \text{Soil Storage})^2}{\text{Rainfall} + 0.8 \times \text{Soil Storage}}$
 - $Q_{5\text{yr-1hr}} = 1.34$ inches
 - $Q_{10\text{yr-1day}} = 6.45$ inches
 - $Q_{25\text{yr-3day}} = 13.20$ inches
 - $Q_{100\text{yr-3day}} = 17.13$ inches
- B. Calculate the Runoff Volume
 - $V = Q \times \text{Project Area (1 ft / 12 in)}$
 - $V_{5\text{yr-1hr}} = 0.10$ ac-ft
 - $V_{10\text{yr-1day}} = 0.50$ ac-ft
 - $V_{25\text{yr-3day}} = 1.03$ ac-ft
 - $V_{100\text{yr-3day}} = 1.34$ ac-ft
- C. Neglecting discharge and using the stage-storage chart and the calculated runoff volumes, the stages can be interpolated.
 - $\text{Stage}_{5\text{yr-1hr}} = 3.6$ ft
 - $\text{Stage}_{10\text{yr-1day}} = 5.5$ ft
 - $\text{Stage}_{25\text{yr-3day}} = 7.52$ ft
 - $\text{Stage}_{100\text{yr-3day}} = 7.98$ ft

Water Quality Calculations

- A. Compute the first inch of runoff from the entire site.
- = 1 inch x Total Area x (1 ft / 12 in)
= 0.08 ac-ft
- B. Compute 2.5 inches times the percentage of imperviousness.
- a. Site Area (SA), for water quality pervious/impervious calculations only
SA= Total Area - (roof + lake)
= 0.81 Acres
- b. Impervious Area (IA), for water quality pervious/impervious calculations only
IA= Site Area(SA) - Pervious Area
= 0.43 Acres
- c. Percentage of imperviousness for water quality
%imp= (IA / SA) x 100%
= 53.24 %
- d. For 2.5 inches times percentage of imperviousness
= 2.5 inches x %imp
= 1.33 inches
- e. Compute volume required for quality detention
= inches to be treated x (total - lake) x (1 ft / 12 in)
= 0.104 ac-ft **(CONTROLS)**
- C. Since 2.5 inches times the percentage of impervious is greater than the first inch of runoff over the entire site, the volume to be treated is:
- Volume to be treated = 0.104 ac-ft
- The Water Quality Treatment is collected and stored in the exfiltration trench and dry retention
WATER QUALITY VOLUME MET AT ELEV. 3.63